

## Validation of Predicted California Bearing Ratio Values from Different Correlations

K Shiva Prashanth Kumar<sup>1</sup>, P M B Raj Kiran Nanduri<sup>2</sup>, Prof.N Darga Kumar<sup>3</sup>

<sup>1,2</sup>(Lecturer, Department of Civil Engineering, Adama Science and Technology University, Adama, Ethiopia)

<sup>3</sup>(Professor, Department of Building and Civil Engineering, Fiji National University, Suva, Fiji)

**ABSTRACT :** In the present study different correlations were used to validate the experimental California Bearing Ratio (CBR) values with predicted values proposed by different investigators. For the present study, 17 samples were collected at different intervals of sub grade soil samples from Modjo to Hawassa, in Ethiopia. From the collected samples, the basic index properties like Liquid Limit (LL), Plastic Limit (PL), Shrinkage Limit (SL), Sieve Analysis, Optimum Moisture Content (OMC) and Maximum Dry Density (MDD) have been evaluated in the laboratory. Finally, the experimental results are validated with the predicted results of CBR. All most all the correlations are slightly validating with the experimental results except Agarwal & Ghanekar [1], Vinod & Cletus [2] and NCHRP [3].

**Keywords -** CBR value, correlation, pavement design, index properties

### I. INTRODUCTION

Soil characterization plays a vital role in early stages of planning and design of an infrastructure projects such as highways, airports, seaports and railways etc., due to the fact that faulty information about the ground characterization can result in high cost and damage to the structures. Most of the pavement design guidelines are based on the assumption that aggregates are important ingredients of pavement structures. However, the availability of good quality aggregates may be a constraint in many instances in the construction. Collection and transport of good quality aggregates from long distances to the construction site may not be economically feasible. Due to the excessive investment and maintenance cost, researchers are striving to introduce appropriate design methods and building materials for cost effective infrastructure development. Sometimes soil characteristics can be improved by the locally available admixtures such as lime, cement, fly ash, stone dust, bitumen, recycled aggregate, glass powder and chemical admixtures so as to meet the design requirements of the intended project. In the recent past, the developed and developing countries are giving priority for development of transport facility so as to have effective connectivity for transporting passengers, goods and other important items for livelihood. Development of road network is regarded as an index of economic, social and commercial progress of a particular country. No region or country can flourish/progress, if it lacks adequate transport facilities and mainly in road network. Highways are the major infrastructures used for transport purpose. Sustainable and cost effective highway construction can be achieved with the help of exact soil sub grade information. In the design of highway pavement, the California Bearing Ratio (CBR) value can be treated as important parameter in the strength assessment of the pavement sub grade. CBR value can be measured directly from the laboratory CBR test. Decision making of proper CBR value to be chosen in the design is always a problem for highway design engineers. CBR value can be affected by the type of soil and different soil index properties. In the present paper, an attempt is made to bring up the correlations of CBR with

soil index properties. These types of correlations can help the designer to choose appropriate CBR value and cross verify the CBR value obtained from the laboratory testing. Along with the soil test data, some of the existing correlations are made use for further improvement of the correlations. Existing correlations for CBR are made use to validate the laboratory CBR values. The existing correlations for CBR were developed based on the soil parameters such as liquid limit, plasticity index and OMC by **Venkatraman et al. [4]**, **Karunaprema and Edirisinghe [5]**.

**Agarwal and Ghanekar [1]** developed a correlation between CBR values with the basic index properties such as Liquid Limit (LL) and Plasticity Index (PI). However, it is felt that exact information about soil LL and PI are essential to make use of the correlations for prediction of CBR. Further for prediction of CBR of a soil, they made a correlation between CBR, OMC and LL also was proposed. Instead, finally they found an improved correlation when they included the optimum moisture content and liquid limit. The correlation is defined as below.

$$\text{CBR} = 2 - 16 \log (\text{OMC}) + 0.07 (\text{LL}) \quad \dots\dots\dots (1)$$

Where OMC = Optimum Moisture Content and LL = Liquid Limit.

**Vinod and Cletus [2]** had proposed a correlation based on liquid limit and gradation characteristics of soils. Based on the result obtained from experimental study on lateritic soils, they suggested a correlation as defined below.

$$\text{CBR} = -0.889 (W_{\text{LM}}) + 45.616 \quad \dots\dots\dots (2)$$

Where  $W_{\text{LM}}$  is modified Liquid Limit and is given by

$$W_{\text{LM}} = \text{LL} \left( 1 - \frac{C}{100} \right) \quad \dots\dots\dots (3)$$

Where LL is liquid limit on soil passing through 425  $\mu\text{m}$  sieve (in percentage) and C is the fraction of soil coarser than 425  $\mu\text{m}$  (percent).

**Patel and Desai [6]** had proposed a correlation between plasticity index, maximum dry density and optimum moisture content.

$$\text{CBR} = 43.907 - 0.093 (\text{PI}) - 18.78 (\text{MDD}) - 0.3081 (\text{OMC}) \quad \dots\dots\dots (4)$$

Where, MDD is Maximum Dry Density in gm/cc, PI is Plasticity Index in percentage and OMC is Optimum Moisture Content in percentage.

**Roy et al [7]** proposed a method for predicting the value of CBR in terms of the optimum moisture content and maximum dry density of a soil. The following relationship as mentioned below

$$\log(\text{CBR}) = \log \left( \frac{Y_{\text{dmax}}}{Y_w} \right) - \log(\text{OMC}) \quad \dots\dots\dots (5)$$

Where,  $\gamma_{dmax}$  or MDD = Maximum Dry Density and  $\gamma_w$  is unit weight of water. Both are in same units (i.e., gm/cc).

**Patel and Desai [6]** had proposed few correlations for alluvial soils to obtain the CBR value from liquid and plastic limit. The equation for CBR as a function of different soil properties by method of regression analysis has been established. The correlations are experienced using basic soil properties for no if samples at 100 m interval and it was checked by few test results obtained CBR values. The correlations are defined below.

$$\text{CBR} = 4.745 - 0.044(\text{LL}) + 0.1508 \quad \dots\dots\dots (6)$$

$$\text{CBR} = 5.176 - 0.028(\text{LL}) - 0.047 (\text{PL}) \quad \dots\dots\dots (7)$$

Where, CBR is California bearing ratio, LL is liquid limit and PL is plastic limit.

The **National Co-operative Highway Research Program (NCHRP) [3]** of United States of America (USA) through the "Guide for Mechanical - Empirical Design of New and Rehabilitated Pavement Structures" had been developed some correlations that clearly describes the relationship between soil index properties and CBR values. An equation was established for soils which contain 12% fines and exhibit some plasticity. For plastic, fine-grained soils, the soil index properties chosen to correlate CBR are the percentage passing No. 200 US standard sieve or 0.075mm size sieve and plasticity index. The suggested equation by NCHRP is shown below.

$$\text{CBR} = \frac{75}{1 + 0.728 (\text{wPI})} \quad \dots\dots\dots (8)$$

Where w is percentage passing No. 200 US sieve (in decimal) and PI is Plasticity Index.

## II. EXPERIMENTAL PROGRAM

The soil samples were collected from 17 different points at each interval of 5m over a longitudinal stretch of 100m of existing sub grade soils between Modzo to Hawassa road, in Ethiopia. These samples were collected and preserved in air tight containers in the laboratory. The selected soil samples were tested for CBR value, optimum moisture content, maximum dry density, particle size distribution (or) grain size distribution, liquid limit, plastic limit, plasticity index and shrinkage limit. These tests were performed according to American Association of State Highway and Transportation Officials (AASHTO) soil code specifications. Almost all the soils are light brown to brown sandy clayey with little gravel. The index properties of soil samples are tabulated in Table 1. The comparison of CBR values predicted from different investigators with experimental values has done.

**Table 1. Laboratory index properties of materials**

S.No	LL	PL	PI	OMC (%)	MDD (gm/cc)	CBR (%)
1	55	26	29	20.0	1.633	2
2	44	25	19	15.0	1.764	5
3	29	20	9	10.0	1.958	21
4	45	29	16	21.0	1.556	4
5	46	31	15	22.5	1.512	3

6	41	24	17	19.0	1.690	4
7	26	18	8	14.0	1.779	10
8	25	22	3	17.5	1.595	18
9	35	27	8	17.0	1.560	9
10	36	30	6	25.0	1.537	8
11	35	32	3	17.0	1.322	18
12	23	21	2	12.0	1.847	7
13	38	30	8	17.0	1.389	4
14	40	30	10	19.0	1.674	9
15	38	35	3	10.0	1.650	18
16	56	43	13	19.5	1.066	17
17	31	25	6	15.0	1.635	8

### III. RESULTS AND DISCUSSIONS

Various investigators have been made to develop suitable correlation between CBR values of compacted soils at optimum moisture content and results of some simple field tests. The CBR values are then evaluated from available correlations given by Agarwal and Ghanekar [1] (Eqn. 1), Vinod and Cletus [2] (Eqn. 2), Patel and Desai [6] (Eqn. 4), Roy et al [7] (Eqn. 5), Patel and Desai [6] (Eqn. 6 and 7) and NCHRP [3] (Eqn. 8). From different investigators, the predicted CBR values were then compared with experimental values reported in Table 1. The predicted values of various equations with experimental values are tabulated in Table 2. The values are further plotted in Fig. 1.

**Table 2. Experimental and Predicted Values of CBR**

S.No	Predicted CBR values (Vinod and Cletus)	Predicted CBR values (Patel and Desai) -1	Predicted CBR values (Roy et al)	Predicted CBR values (Patel and Desai) - 2	Predicted CBR values (Patel and Desai) - 3	Predicted CBR values (NCHRP)	Experimental Values of CBR
1	7.477	4.380	3.329	6.245	2.414	5.244	2
2	10.411	4.390	2.697	6.579	2.769	8.441	5
3	39.170	3.217	1.995	6.485	3.424	39.117	21
4	14.812	6.727	3.3308	7.138	2.553	9.122	4
5	9.629	7.184	3.467	7.395	2.431	7.790	3
6	13.540	4.732	3.273	6.560	2.900	7.474	4
7	28.742	5.439	2.538	6.315	3.602	20.713	10
8	27.836	8.282	2.845	6.962	3.442	44.146	18
9	23.213	8.628	2.703	7.276	2.927	18.352	9
10	16.812	6.781	3.916	7.685	2.758	18.286	8
11	33.792	13.563	2.290	8.030	2.692	48.512	18
12	36.005	5.337	2.259	6.899	3.545	53.835	7
13	22.306	11.839	2.407	7.597	2.702	19.461	4
14	15.034	5.685	3.242	7.509	2.646	14.564	9
15	35.143	9.56	1.681	8.351	2.467	54.693	18
16	28.191	16.670	2.118	8.765	1.587	21.095	17
17	31.285	8.022	2.5	7.151	3.133	31.279	8

From the previous investigators, it has been concluded that few relationships made by basic index properties of soil samples are matching with the laboratory test values. But the reliability of the predicted CBR values basically depends on the input parameters which we used in substituting already developed correlations by different investigators. The parameters such as Liquid Limit, Plasticity Index, Optimum Moisture Content, Maximum Dry Density and particle size or grain size values are playing vital role in predicted CBR value proposed by different investigators.

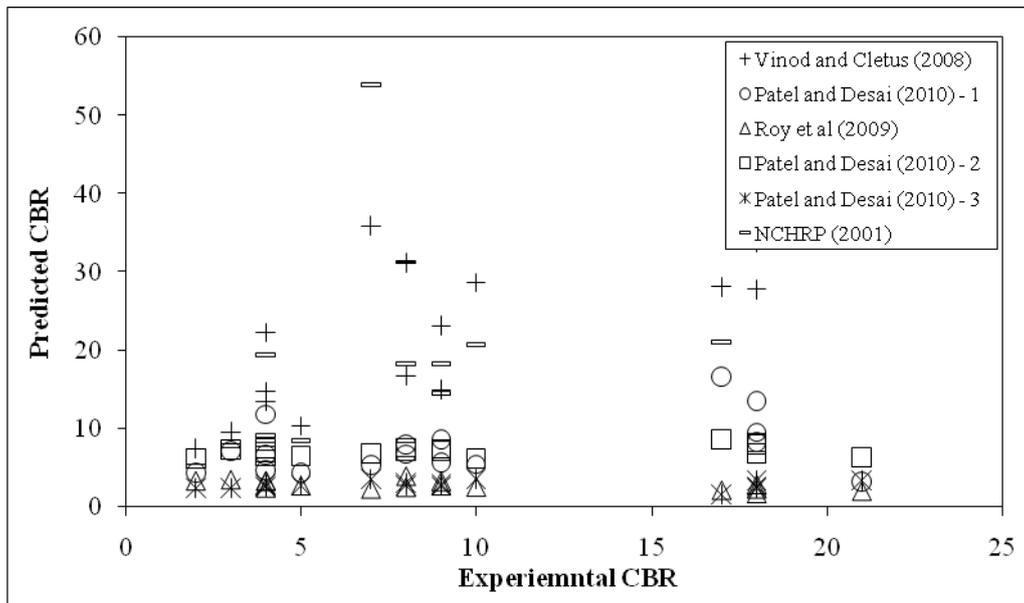
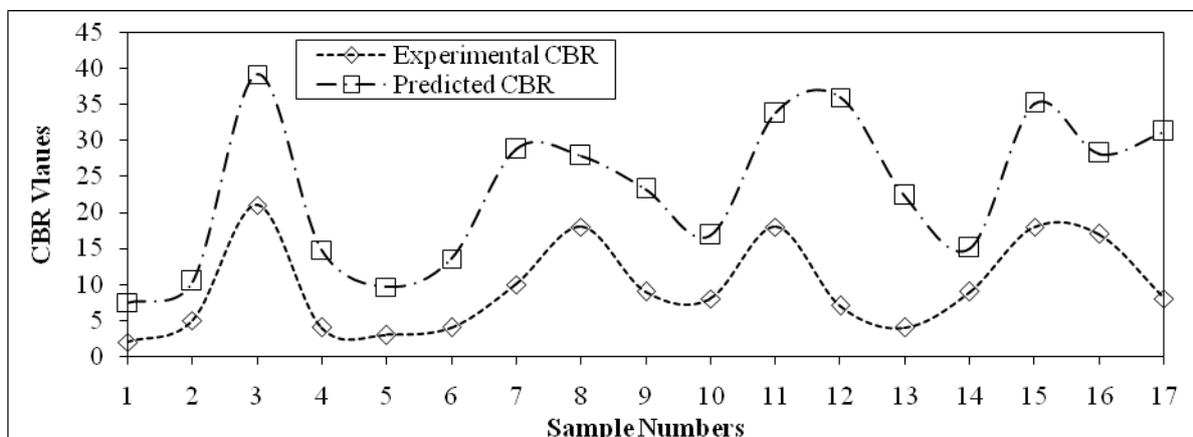


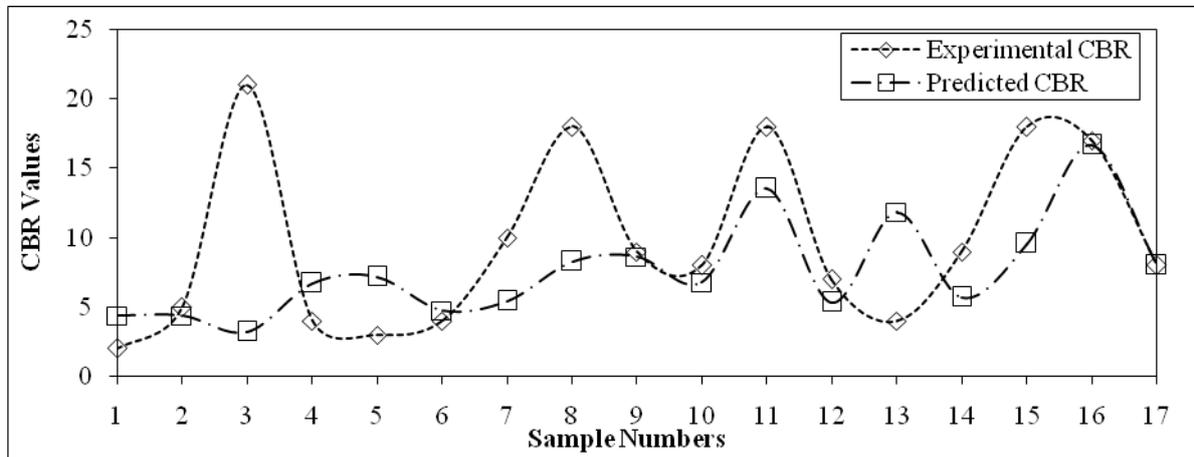
Fig. 1. Comparison of Experimental and Predicted CBR values by different investigators

Fig. 1 shows the comparison of predicted and experimental CBR values suggested by different investigators. From the above figure it is clearly observed that the predicted value of CBR from Vinod and Cletus [2] shows wide range of divergence from all other equations of the soils reported in Table 1. Almost all the equations are validating the predicted values with obtained experimental values. The predicted value of CBR from Vinod and Cletus [2] showing very high values of CBR compared with obtained experimental CBR. The remaining all investigators proposed equations are in moderate range of predicted CBR values as compared to experimental CBR. From the above figure it is clearly seen that the points of predicted and experimental CBR values are merging together (i.e., the values of experimental and predicted values are almost similar) except CBR values from Vinod and Cletus and NCHRP [3].



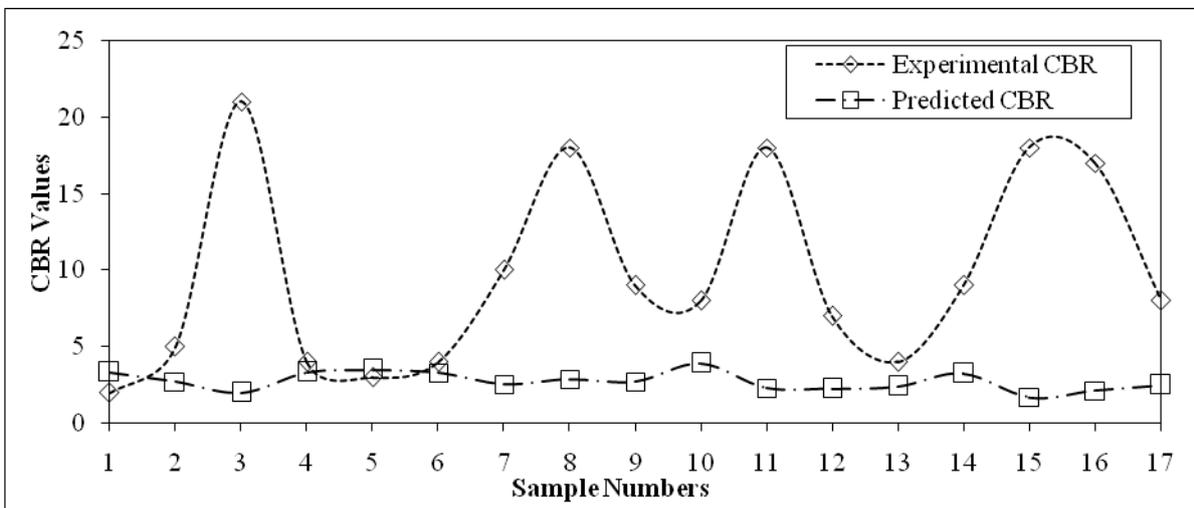
**Fig. 2. Comparison of Experimental and Predicted values of CBR from Vinod and Cletus.**

Fig. 2 shows the validation of CBR values between experimental and predicted from Vinod and Cletus. The values are not at all showing the similarity between experimental and predicted CBR. This may be due to the variation of basic index properties for different soils such as liquid limit and percentage fractions coarser than 425  $\mu\text{m}$ . The values compared to the experimental CBR and predicted CBR values from Vinod and Cletus [2] are very higher.



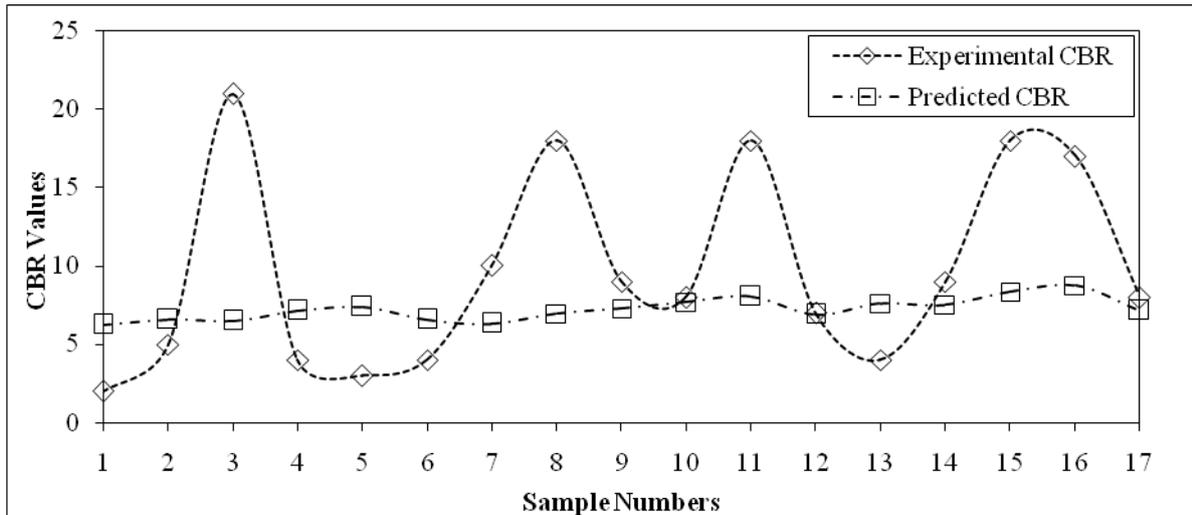
**Fig. 3. Comparison of Experimental and Predicted values of CBR from Patel and Desai -1.**

Fig. 3 shows the validation of experimental and predicted values of CBR from Patel and Desai [6] - 1. From the figure it is observed that the predicted CBR values are initially stated high and finally it is almost matching and moving together. The ranges of index properties such as plasticity index, optimum moisture content and maximum dry densities for the soils which we considered for laboratory testing is almost similar with investigated correlation ranges.



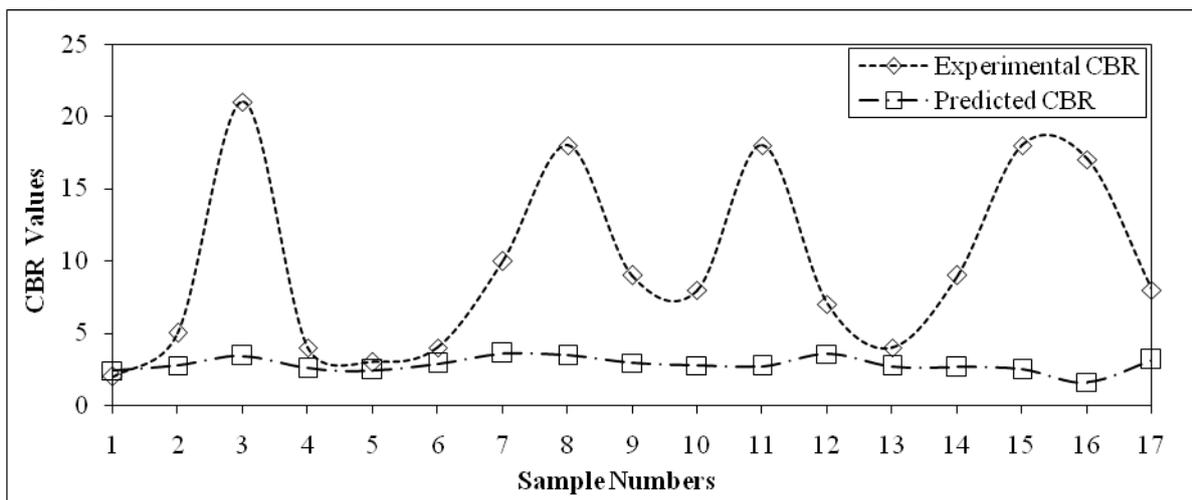
**Fig. 4. Comparison of Experimental and Predicted values of CBR from Roy et al.**

Fig. 4 shows the validation of experimental and predicted values of CBR from Roy et al [7] From the figure it is seen that the predicted value of CBR is initially more than experimental values. This may be due to the variation in parameters such as maximum dry density, unit weight and optimum moisture content. At sample number 4, 5 and 6 are matching the experimental and predicted CBR values. But there is no proper trend between the experimental and predicted CBR values.



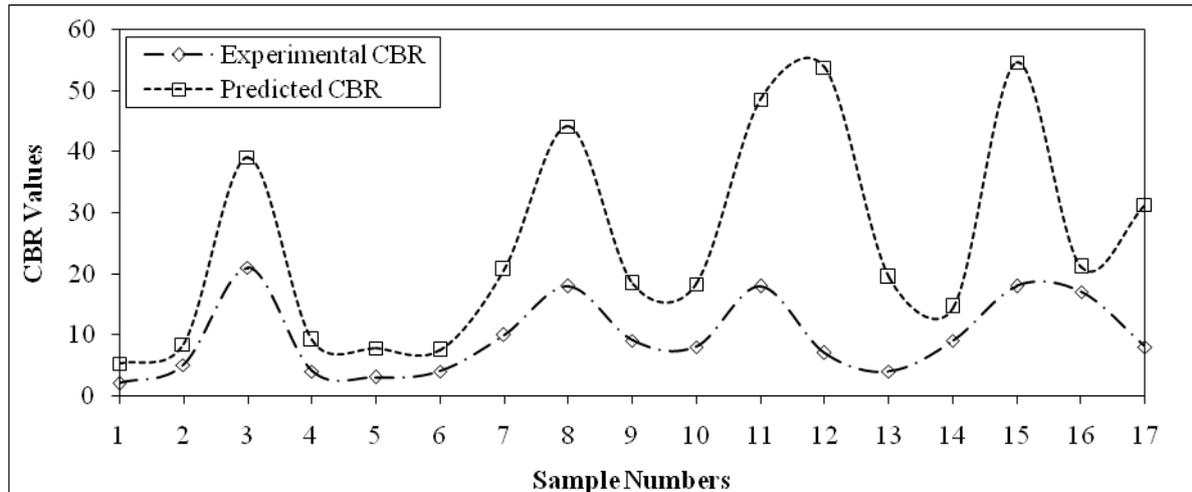
**Fig. 5. Comparison of Experimental and Predicted values of CBR from Patel and Desai -2.**

Fig. 5 shows the validation of experimental and predicted values of CBR from Patel and Desai [6] - 2. From the figure the values of predicted CBR is initially more than experimental CBR value. The values are matching with at sample number 10, 12 and 17. But it is not at all showing the similarity or proper trend along with the experimental data. This may be due to the variation of properties such as liquid and plastic limits of various soil samples.



**Fig. 6. Comparison of Experimental and Predicted values of CBR from Patel and Desai -3.**

Fig. 6 shows the validation of experimental and predicted values of CBR from Patel and Desai [6] -3. From the figure, it is observed that the predicted values are not matching with the experimental values. At initial sample the experimental value of CBR coincide with the predicted value from Patel and Desai [6] - 3. The experimental values are higher than predicted values. This may be the variation in liquid and plastic limits of various soil samples. But finally, it is not following the trend as with the experimental value.



**Fig. 7. Comparison of Experimental and Predicted values of CBR from NCHRP.**

Fig. 7 shows the variation of experimental and predicted values of CBR from NCHRP [3]. From the figure it is clearly observed that almost both the values are following the trend but those values are not exactly matching together. The predicted values are little higher than the experimental values. Finally proper trend of the curve is following with slight deviation in predicted values.

#### IV. CONCLUSION

Experimental values of 17 samples of various index properties are given as input to validate the equations with predicted CBR values. Several tests have been conducted to measure the CBR and index properties of soils were performed. Finally few conclusions were brought out.

- 1) The results obtained from Agarwal and Ghanekar [1] has no way matching with the experimental as well predicted CBR values.
- 2) All most all the equations are moderately validating with different samples with the experimental values with predicted values of CBR except Vinod and Cletus [2].
- 3) From NCHRP [3], the experimental and predicted values are following the trend but the values are not matching exactly.

#### V. Acknowledgements

First of all, I would also thank to my colleagues Mr. Shumi and Mr. Esaias from Adama Science and Technology University (ASTU), Adama, Ethiopia, for their assistance in the laboratory.

#### REFERENCES

- [1] Agarwal, K.B.; and Ghanekar, K.D. (1970). Prediction of CBR from Plasticity Characteristics of Soil. Proceeding of 2<sup>nd</sup> South-east Asian Conference on Soil Engineering, Singapore. June 11-15, Bangkok: Asian Institute of Technology, 571-576.
- [2] Vinod, P.; and Cletus Reena. (2008). Prediction of CBR value of Lateritic Soils using Liquid Limit and Gradation Characteristics Data, Highway Research Journal, Vol. I, No. 1, 89-98.
- [3] NCHRP. (2003). Guide for mechanistic-empirical design of new and rehabilitated pavement structures. National Co-operative Highway Research Program Transportation Research Board National Research Council, Illinois 61820.
- [4] Venkataraman, T.S.; Samson, M.; and Ambili, T.S. (1995). Correlation between CBR and Clegg Impact Value, Proc. Nat. Sem. On emerging trends in Highway Engineering, Centre for Transportation Engineering, Bangalore, Vol. I, 25.1-25.5.
- [5] Karunaprema, K.A.K. and Edirisinghe, A.G.H.J. (2002). A Laboratory study to establish some useful relationship for the case of Dynamic Cone Penetration, Electronic journal of Geotechnical Engineering. Vol.7.
- [6] Patel, R. S.; and Desai, M.D. (2010). CBR Predicted by Index Properties of Soil for Alluvial Soils of South Gujarat, Indian Geotechnical Conference, Proc. IGC, Vol. I, 79-82.
- [7] Roy, T.K.; Chattopadhyay, B.C.; and Roy, S.K. (2009). Prediction of CBR from Compaction Characteristics of Cohesive Soil,

- Highway Research Journal, July-Dec., 77-88.
- [8] Chandrasekhar, B.P.; Vinayak Rao, V.R.; and Prasada Raju, G.V.R. (2003). A comprehensive study of soil CBR assessment through Clegg's Impact test, Field and Laboratory CBR test, *Indian Highways*, Vol.31, No 7, 39-45.
- [9] Venkatasubramanian, C.; and Dhinakaran, G. (2011). ANN model for predicting CBR from index properties of soils, *International Journal of Civil and Structural Engineering*, Volume 2, No.2.
- [10] American Standard Test Method. (1992). Standard Test Method for CBR (California Bearing Ratio) of Laboratory- Compacted Soils. United States of America, ASTM Designation D1883-92.
- [11] Sister, N.W.; and Powell, W.D. (1976). Design Practice for Bitumen Pavements in the United Kingdom, *Proceedings of 6th International Conference on Structural Design of Asphalt Pavements*, Volume 1.