American Journal of Engineering Research (AJER)
e-ISSN: 2320-0847 p-ISSN : 2320-0936
Volume-9, Issue-4, pp-26-32
www.ajer.org
Research Paper Open Access

Effect of OPC-SAW Dust Ash Composite on the Compressive Strength of Concrete under Prolonged Curing

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ABSTRACT: *The soaring cost of construction materials in Nigeria and beyond has constrained researchers to continue to seek ways of reducing the cost of building projects. This research work investigated the effect of partial replacement of ordinary portland cement (OPC) with saw dust ash (SDA) on the compressive strength of concrete under prolonged curing. 100 concrete cylinders of diameter 100mm and height 200mm cylindrical mould were produced at percentage of OPC replacement with SDA of 10%, 20%, 30% and 40% and crushed to obtain their compressive strength value at 7, 14, 21, 28 and 56 days of curing. The compressive strength of the concrete cylinder increased as the OPC replacement with SDA increases till 10% replacement and then started to decrease as the percentage replacement was increased beyond this point. This result was based on a primary data collected. The optimum percentage of replacement of OPC with SDA was 10%. Mathematical models were developed to predict the compressive strengths of OPC – SDA blended composites using non-linear regression analysis employing exponential function. The model values obtained from the various equations adequately correlated with the laboratory values. The results supported earlier researches on saw dust ash as a partial replacement for cement in concrete production and concludes that OPC-SDA blended composite could be good for civil engineering works with good quality control.*

KEYWORDS: *Composite; concrete; compressive strength; saw dust ash; Pozzolana;*

Date of Submission: 20-03-2020 Date of Acceptance: 06-03-2020

I. INTRODUCTION

The growing concern of high cost of construction materials as regards the need to provide adequate and affordable accommodation for the teaming population of Nigerians both in the urban and suburban areas has challenged engineers to seek and develop new materials relying on locally available ingredients. In a bid to reduce the cost of building projects suitable and more affordable materials could be used as partial replacement for cement to achieve this purpose. Agricultural by-products regarded as waste in technologically under developed societies could be harnessed in this regard [1]. Blended cements are currently used in many parts of the world [2], because they have been found to meet most of the requirements of durable concrete. Calcium hydroxide [$Ca(OH)_2$] is one of the hydration products of Portland cement which substantially causes the deterioration of cement composites. When a pozzolana is blended with portland cement, it reacts with the lime to produce additional calcium-silicate-hydrate (C-S-H) which is the main cementing compound. In essence the pozzolana reduces the quantity of lime and increase the quantity of C-S-H. The cementing quality is enhanced when pozzolana is blended in suitable quantity with portland cement [3].

Agricultural by-products have been used in the production of blended cement composites.

[4] investigated the use the saw dust ash (SDA) as partial replacement for ordinary Portland cement in sandcrete blocks and inferred that up to 10% SDA replacement is adequate for use in sandcrete blocks for non-load bearing wall in buildings. [5] on the replacement of cement with saw dust ash (SDA) stated that up to 10% SDA substitution is allowed at maximum and 5% substitution is adequate to enjoy maximum benefit of strength gain.[6] successfully investigated the potentials of using groundnut husk ash as partial replacement for ordinary portland cement in concrete. [7] studied the strength of binary blended cement composite containing Pawpaw Leaf Ash. [8], [32] investigated the strength variation of ordinary portland cement (OPC) - Rice Husk Ash (RHA) - Saw Dust Ash (SDA) with percentage RHA- SDA and confirmed their usability as partial replacement for OPC. [9] researched on the compressive strength of ternary blended cement sandcrete containing Coconut

Husk Ash (CHA) and Plantain Leaf Ash (PLA) and inferred that it could be used for various civil engineering and building works where early strength is not a major requirement. [10] investigated the properties of blended cement mortar, concrete and stabilized earth made from OPC and Corn Cob Ash and recommended that it can serve as replacement for OPC in the production of cement composite. [11] further studied the characteristics of laterite bricks and blocks stabilized with Corn Cob Ash as laterite stabilizer for block making. [12], [31] highlighted the potentials of Coconut Husk Ash, Corn Cob Ash and Peanut Shell Ash as good pozzolanas. [13] successfully investigated the pozzolanicity of Bamboo Leaf Ash. [14] found that sugar industry solid waste such as Sugar cane Straw Ash has pozzolanic activity derived from its high content of armophous silica. Some other researchers have also confirmed the possibility of using sugar industry waste as pozzolans [15], [16]. Many other researchers have confirmed Rice Husk Ash a pozzolanic material that can be used to partially replace OPC in making cement composites [17], [18] and [19]. A number of researchers have also found prospects in using blended cement made with Sawdust Ash [20], [21] and [22]. [23] suggest that soil, climatic and geographical conditions could affect the physical and chemical properties and consequently the pozzolanicity of agricultural products.

Nigeria is a country with vast timber resources mostly in the southern region. There is and will continue to be large quantity of saw dust produced from the various timber factories in the region, hence the relevance of the research. The utilization of saw dust ash as a pozzolanic material would add commercial value to otherwise waste product. The objective of the research is to ascertain the suitability of Saw Dust Ash as a partial replacement to OPC for use in concrete production.

II. MATERIALS AND METHODS

Ordinary Portland cement of Ibeto brand was used as the hydraulic binder. Saw dust was collected from Timber Shade, Maryland Enugu. Saw dust ash was produced by incinerating saw dust in a purpose-made industrial incinerator at temperature of between 650^0c and 800^0c. The temperature was measured by the use of a pyrometer. The ash was sieved and large particles retained on the 150µm sieve were used for the research. No grinding or any special treatment to improve the ash quality and therefore enhance its pozzolanicity was applied because simplicity and affordability were of main concern in the research. The coarse aggregate used for the research was crushed granite of norminal size 20mm obtained from Abakaliki, Ebonyi State. Fine aggregate used was sharp river sand collected from Nyama River in Enugu. Pipe borne water fit for drinking was used for the research.

Moisture content test was conducted to determine the amount of water present in the fine aggregate sample in conformity with [24]. The formula used to calculate the moisture content (w) is shown in Equation (1).

$$w = \frac{M2 - M3}{M3 - M1} \, x \, 100\% \qquad (1)$$

Where; $M1$ is the mass of the container (in grams), $M2$ is the mass of the container and wet sample (in grams) and $M3$ is the mass of the container and oven-dried sample (in grams).

Specific gravity of the various aggregates were determined in accordance with [25], Equation (2) shows the formula used to calculate it.

$$\rho_d = \frac{D}{A-(B-C)} \qquad (2)$$

Where; ρ_d is Specific gravity, A is the mass of the saturated and surface-dry sample (in grams), B is the mass of vessel containing sample and filled with water (in grams), C is the mass of the vessel filled with water only (in grams), D is the mass of the oven-dried sample (in grams).

Bulk density of fine, coarse and the compacted fresh concrete was conducted according to [26], Equation (3) was used to calculate the density.

$$\rho = \frac{W2-W1}{V} \qquad (3)$$

Where; ρ is bulk density (in Kg/m^3), W1 is the mass of empty cylinder (in Kg), W2 is the mass of cylinder and sample (in Kg) and V is the volume of the cylinder (in m^3).

Particle size distribution test (sieve analysis) was conducted in accordance with [27].

A pozzolanicity test was carried out for the SDA. It consists of mixing a given mass of the ash with a given volume of calcium hydroxide solution [$Ca(OH)_2$] of known concentration and titrating samples of the mixture against hydrochloric acid solution of known concentration at time intervals of 30, 60, 90 and 120 minutes using phenolphtaline as indicator at normal temperature. The titre value was observed to reduce with time, confirming the ash as pozzolana that fixed more and more of the calcium hydroxide, thereby reducing the alkalinity of the mixture.

A slump test was carried out to determine the workability of the fresh concrete. This was done in conformity with [28]. Also compressive strength test for the various cylindrical specimens was conducted in accordance with [29], the formula is shown Equation (4).

$$Compressive\ Strength = \frac{Crushing\ load\ (N)}{Cross\ sectional\ \ area\ (mm^2)} \tag{4}$$

A common mix ratio of 1:2:4 (blended cement: sand: granite) was used for the concrete cylinders. Batching was by weight and a constant water cement ratio of 0.55 was used. A mix ratio of 1:2:4 and a water/cement (w/c) of 0.55 were selected because they do not constitute the main focus of the research and they were kept constant throughout the test. Moreover, mix ratio 1:2:4 is among the most common in concrete mixes. Mixing was done manually, SDA was first thoroughly blended with OPC at the required proportion and the homogenous blend was then mixed with the fine aggregate, coarse aggregate also at the required proportions. Water was then added gradually and the entire concrete was mixed thoroughly to ensure homogeneity.

Eighty (80) concrete cylinders of 100mm diameter and 200mm height cylindrical mould were produced at percentage OPC replacement with SDA of 10%, 20%, 30% and 40%. Twenty concrete cylinders with 100% OPC were also produced to serve as controls. This gives a total of 100 concrete cylinders. All the concrete cylinders were cured by immersion. Four concrete cylinders for each percentage replacement of OPC with SDA and the control were tested for saturated surface by bulk density and crushed to obtain their compressive strengths at 7, 14, 21, 28 and 56 days of curing. Average values of concrete compressive strengths for various curing ages and percentages of OPC replacement with SDA were obtained and presented in tables and graphs. Empirical models were developed to predict the compressive strengths of OPC – SDA blended composites using polynomial regression analysis.

III. RESULTS AND DISCUSSION

The resultant Saw Dust Ash (SDA) had a bulk density of 820kg/m^3, specific gravity of 2.05 and fineness modulus of 1.89. .The ibeto brand of ordinary portland cement (OPC) had bulk density of 1650kg/m^3 and specific gravity of 3.13; river sand free from debris and organic materials had bulk density 1590kg/m^3 and specific gravity 2.66 and fineness modulus of 2.80; crushes granite of 20mm norminal size with bulk density of 1550kg/m^3 and specific gravity of 2.77 and water free from organic impurity. The fine aggregate belonged to zone 3 based on [30], grading limits for fine aggregate. The moisture content of the fine aggregate was found to be 14.3%. Although the quantity of water in the concrete mix has a substantial effect on the compressive strength, this effect cancelled out because the same fine aggregate was used in casting the cubes for both the control (ie 0% replacement) and the various percentages of SDA replacement of OPC.

3.1 POZZOLANICITY TEST CALCULATIONS AND RESULTS

Reacting solution

$H_2SO_4 + Ca(OH)_2 \rightarrow CaSO_4 + 2H_2O$

$$\text{Concentration in mol/dm}^3 = \frac{\text{Reacting mass}}{\text{Molar mass}}$$

Determining of Molar Mass

H_2SO_4 = 98g; $Ca(OH)_2$ = 74g.

Determination of Reacting Mass

For H_2SO_4

Weight of Beaker = 103.4g

Weight of Breaker + Acid = 155.6g

Reacting Mass = 52.2g/dm^3

For $Ca(OH)_2$

Weight of Beaker = 103.4g

Weight of Breaker + Base = 120.8g

Reacting Mass = 17.4g/dm^3

Concentration of Acid, H_2SO_4 in mol/dm^3

$$\frac{\text{Reacting mass}}{\text{Molar mass}} = 0.5533\text{mol/dm}^3$$

Concentration of Base, Ca $(OH)_2$ in mol/dm^3

$$\frac{\text{Reacting mass}}{\text{Molar mass}} = 0.235\text{mol/dm}^3$$

For Saw Dust Ash Solution

Weight of plate pan = 40.8g

Weight of plate + SDA = 53.6g

Reacting Mass = 12.8g

Volume of Water that formed the solution = 25ml

Table 1: EXPERIMENT TIMING TABLE FOR SDA SOLUTION

	30mins		60mins		90mins		120mins	
	1st Titration	2nd Titration	1st Titration	2nd Titration	1st Titration	2nd Titration	1st Titration	2nd Titration
Final burette reading (cm^3)	460	460	11.3	23.4	5.20	5.20	43.8	48.0
Initial burette reading (cm^3)	10.5	01.00	0.8	13.0	0.00	0.00	38.5	43.8
Volume of acid used ($cm^{3)}$)	10.9	11.50	10.5	10.4	5.20	5.20	5.30	4.20
Average	$11.2cm^3$		$10.45cm^3$		$5.20cm^3$		$4.75cm^3$	

Determination of the Concentration of the SDA Mixture

$$\frac{C_B V_B}{C_A V_A} = \frac{nB}{nA}$$

Where

C_B = concentration of base
V_B = volume of base
C_A = concentration of acid
V_A = volume of acid
nB = molar number of base
nA = molar number of acid

For 30 minutes

$C_B = \frac{C_A V_A \times nB}{V_B \times nA}$

$= 0.239 mol/dm^3$

For 60 minutes

$C_B = 0.223\ mol/dm^3$

For 90 minutes

$C_B = 0.111\ mol/dm^3$

For 120 minutes

$C_B = 0.101\ mol/dm^3$

The titre value was observed to reduce with time, confirming saw dust ash (SDA) as pozzolanic properties that fixed more and more of the calcium hydroxide with time, thereby reducing the alkalinity of the mixture.

The particle size of Saw Dust Ash was much more coarse than that of the OPC. This is because the ash was deliberately not pulverized to finer particles. No further grinding or processing was carried out on the ash retained on the 150µm sieve that was used for the research. This is an indication that a higher value of compressive strength can be achieved if the particles were finer thereby increasing the pozzolanicity. The SDA test confirmed SDA as a pozzolana since it fixed some quantities of lime over time thereby reducing the alkalinity of the mixture as reflected in the smaller titre value over time compared to the blank titre. The compressive strength of the blended cement concrete produced with OPC and SDA are shown in the Table 2.

Table 2: Summary of the Mean Strength Results

MIX ID	% SDA	Mean Strength @ Age of Crushing (N/mm^2)				
		7 days	14 days	21 days	28 days	56 days
Mix 1	0	18.91	23.94	28.50	30.04	32.08
Mix 2	10	16.57	22.33	28.50	32.97	34.15
Mix 3	20	15.86	19.60	23.11	25.22	28.70
Mix 4	30	14.25	18.70	20.92	22.11	25.60
Mix 5	40	12.87	15.08	18.96	20.87	22.51

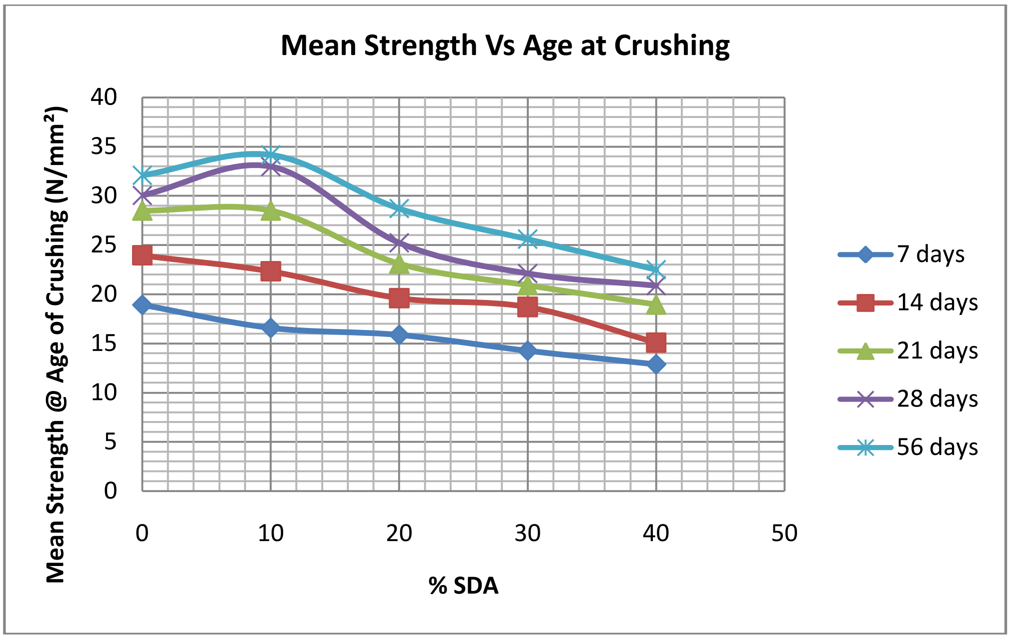

Figure 1: Graph of Mean Strength Result against Percentage Replacement.

From Table 2, the compressive strength values of OPC-SDA blended cement concrete composite at all percentage replacement of OPC with SDA were much lower than the control values at 7 days, but increased to become comparable to and even greater than the control values at 28 to 56 days of curing. This trend is as a result of the low rate of pozzolanic reaction at early ages of curing. The silica from the pozzolana reacts with lime produced as by-product of hydration of OPC to form additional calcium-silicate-hydrate (C-S-H) that increases the binder efficiency and the corresponding strength values at later days of curing.

The 28-day and 56-day strength values for 10% SDA replacement were more than the control values (ie 0% SDA replacement), representing about 109% and 106% of the control strength value respectively. Beyond this point, the compressive strength value began to decline as the percentage of SDA increased. This pattern shows that the optimum percentage replacement of OPC with SDA is 10% since it yielded the highest strength value at 21, 28 and 56 days of curing even more than the control strengths. It can also be observed from Figure 1 that the strength across all percentages of replacement increased as the period of curing increased. This implies that if the samples were cured beyond 56 days, there is a potential for an improved strength. The workability and density of the concrete cylinders decreases with an increase in the percentage of Saw Dust Ash (SDA) in the mix because the slump at the control was higher than other mixes. The results suggest that with good quality control of the concreting process, up to 10% of OPC replacement with SDA could be used for general reinforced concrete works.

3.2 MATHEMATICAL MODELS FOR PREDICTING COMPRESSIVE STRENGTH OF CONCRETE MADE WITH OPC-SDA BLENDED CEMENT COMPOSITE

Mathematical models were developed to predict the compressive strengths of OPC – SDA blended composites using non-linear regression analysis employing exponential function. The models can be easily used, adaptable and the predictions were quite sufficient.

The model equations for predicting the compressive strength of concrete made with OPC-SDA blended cement composite are presented in this section.

$\beta_7 = 18.69e^{-0.00919\lambda}$ R = 0.9909 (5)

Where β_7 is the 7-day compressive strength in N/mm2 is, λ is the percentage replacement of OPC with SDA and R is coefficient of correlation.

$\beta_{14} = 24.53e^{-0.01103\lambda}$ R = 0.9775 (6)

$\beta_{21} = 29.65e^{-0.01125\lambda}$ R = 0.9614 (7)

$\beta_{28} = 32.4e^{-0.0113\lambda}$ R = 0.8848 (8)

$\beta_{56} = 34.53e^{-0.00996\lambda}$ R = 0.9129 (9)

The model values obtained from the various equations adequately correlated with the laboratory values. It can therefore be inferred that the model equations are suitable for predicting the compressive strength of concrete made with OPC-SDA blended cement composite under prolonged curing.

IV. CONCLUSION AND RECOMMENDATION

The compressive strength of the concrete cylinder increased as the OPC replacement with SDA increases till 10% replacement and then started to decrease as the percentage replacement was increased beyond this point. The workability and bulk density of the concrete decreases with increase in the percentage of Saw Dust Ash in the mix. There is an increase in the compressive strength with age irrespective of the proportion of SDA to OPC in the mix. . The model values obtained from the various equations adequately correlated with the laboratory values. This implies that the model equation can sufficiently be used to predict the compressive strength of OPC-SDA blended cement concrete. The strength variation of OPC-SDA composite suggests that with good quality control of the concreting process, up to 10% OPC replacement with SDA could be suitable for general reinforced concrete works, 20% to 40% for minor to plain concrete works.

There is a need to study the chemical and physical properties of the various tree species from which the ashes used is gotten. This will show the difference in composition and pozzolanic activity if any, among the ashes of different tree species.

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Ugwu Juliet Nneka,etal." Effect of OPC-SAW Dust Ash Composite on the Compressive Strength of Concrete under Prolonged Curing." *American Journal of Engineering Research (AJER)*, vol. 9(04), 2020, pp. 26-32.